2897 Adeline Dr

Building

Directions Save Nearby Send to phone Share

2897 Adeline Dr, Burlingame, CA 94010

Photos

Map data ©2025 , Map data ©2025 20 ft

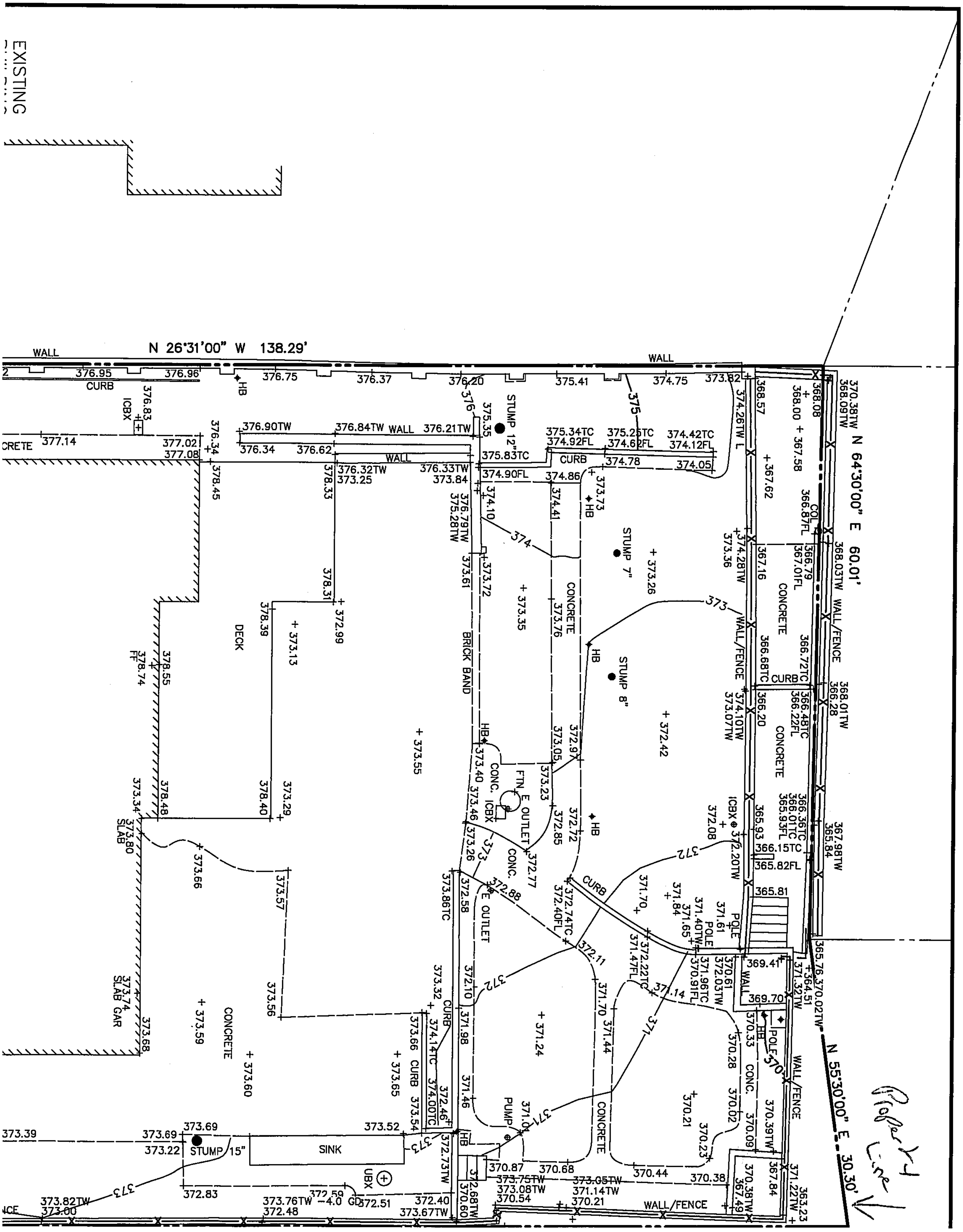
2896 Hillside Dr

Building

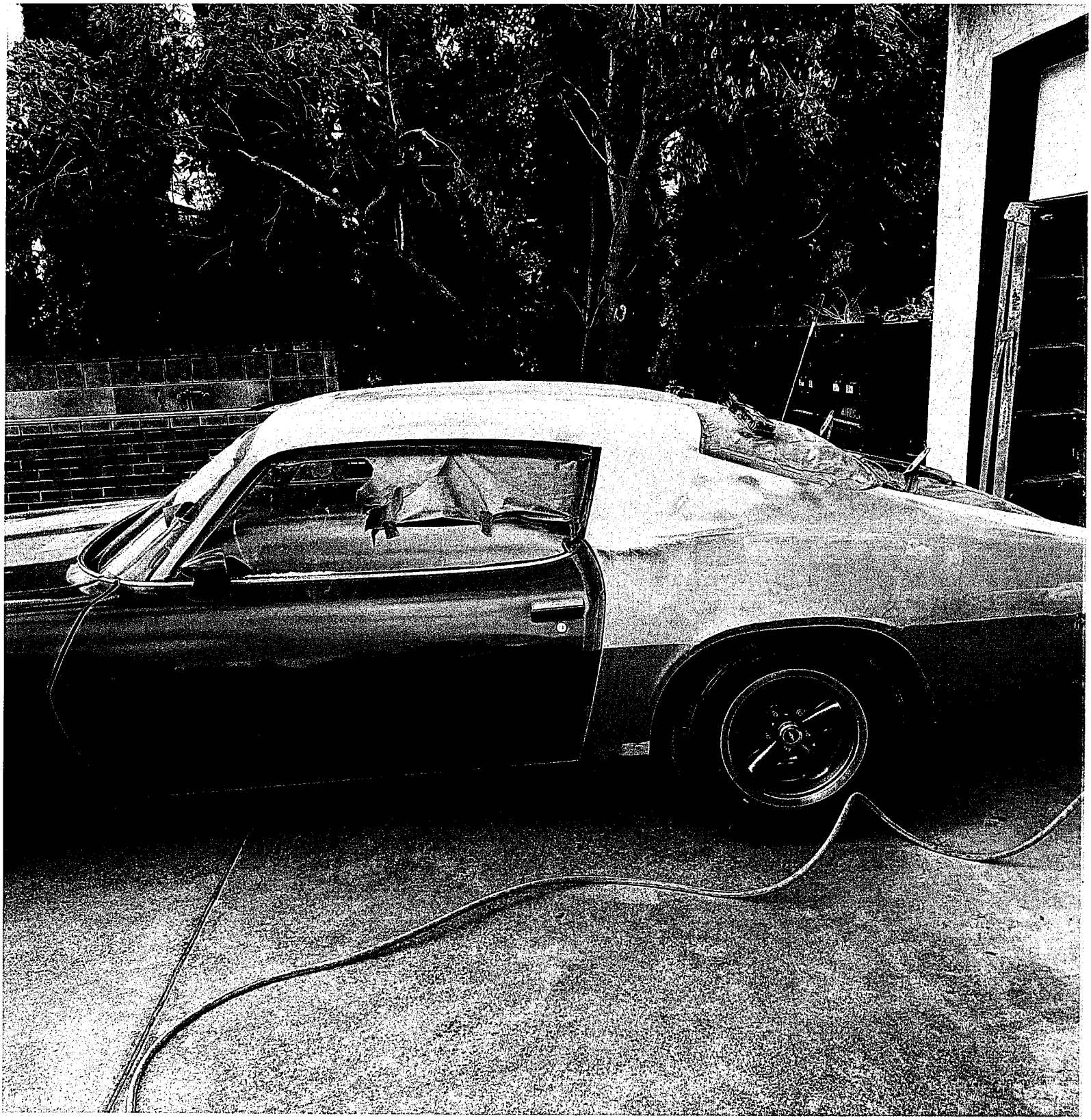
Directions Save Nearby Send to phone Share

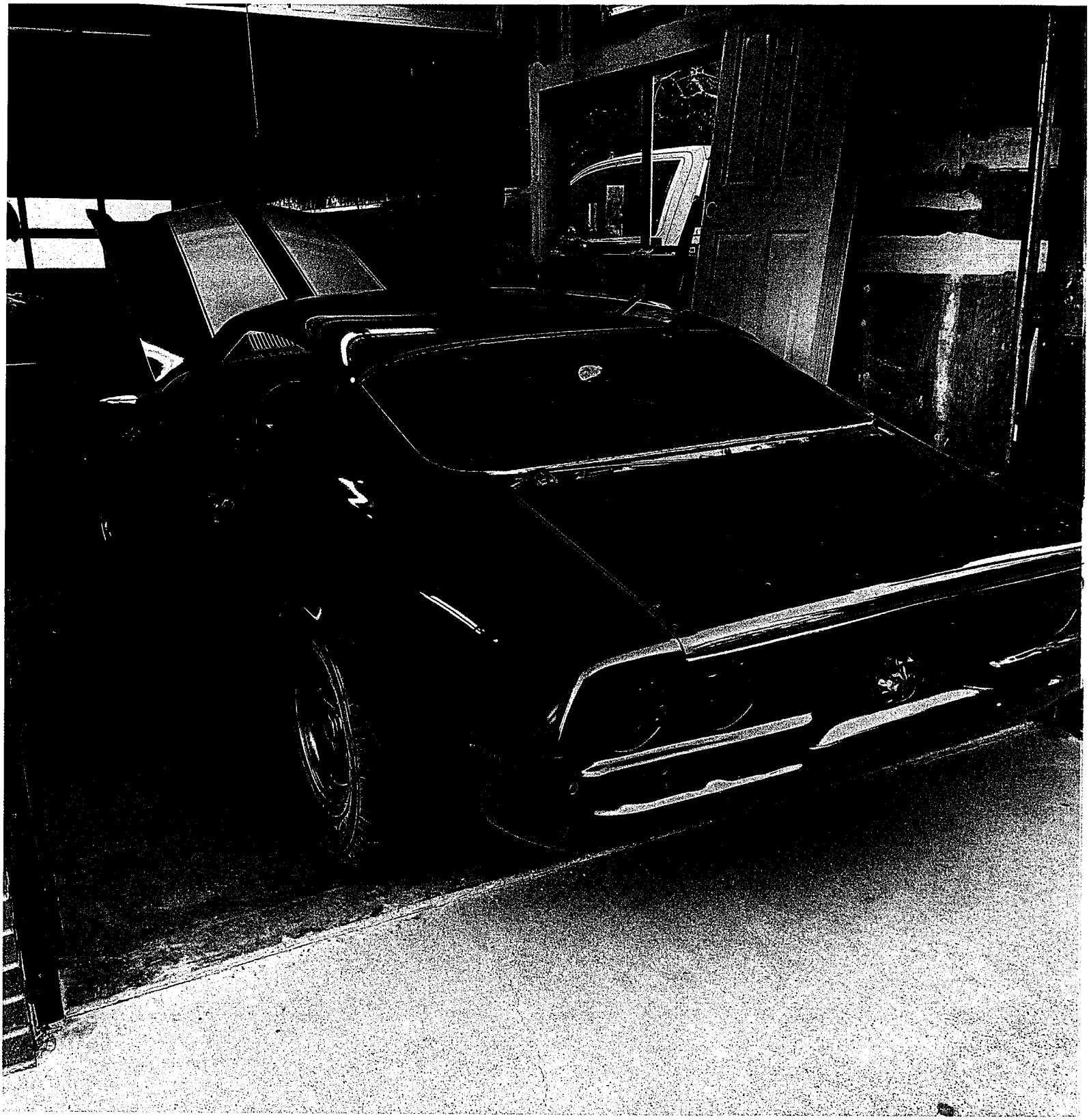
2896 Hillside Dr, Burlingame, CA 94010

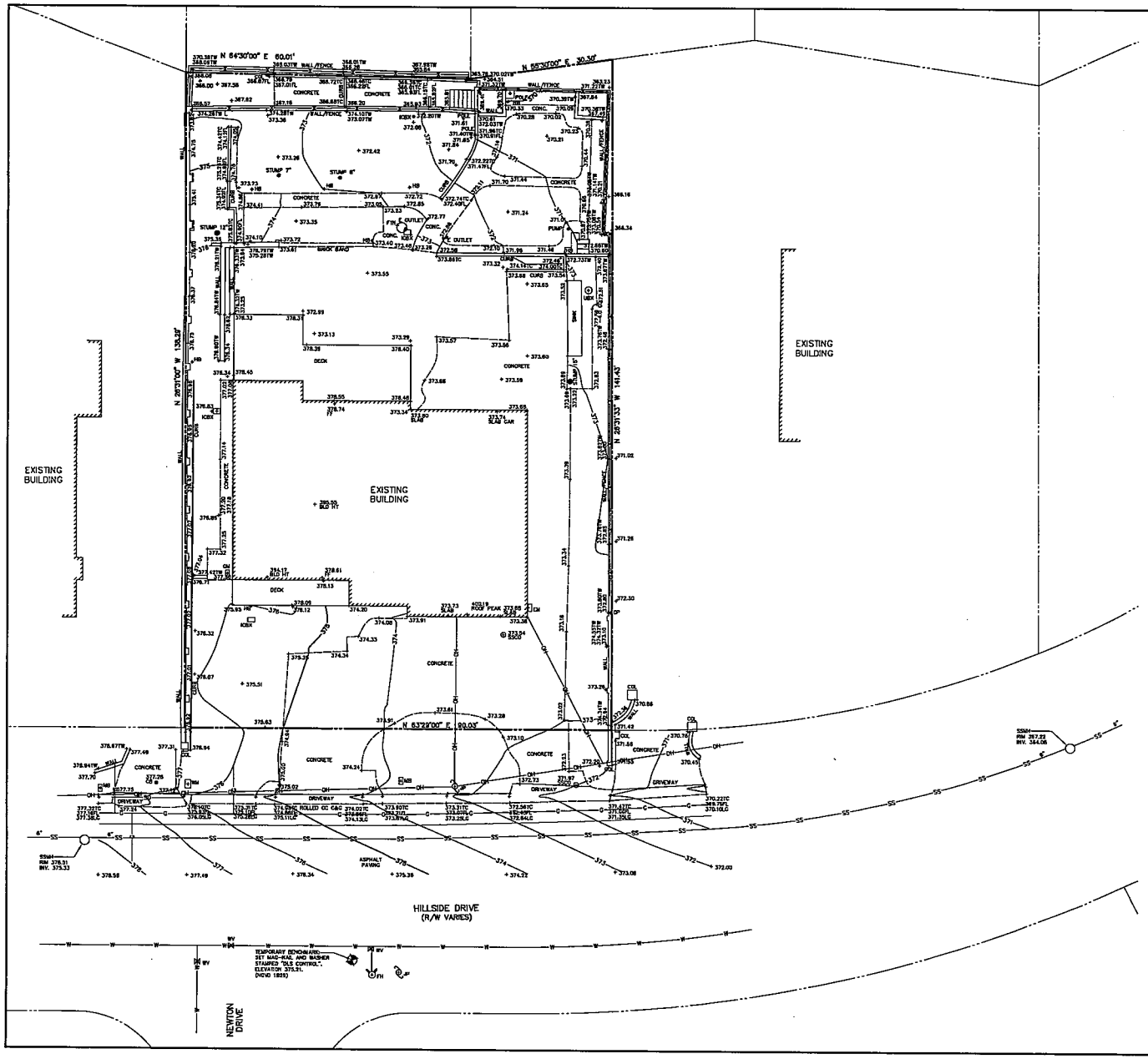
Photos


EXISTING
WALL
N 26°31'00" W 138.29'
N 64°30'00" E 60.01'
N 55°30'00" E 30.30'
CURB
ICBX
DECK
CONCRETE
BRICK BAND
STUMP 12"
STUMP 7"
STUMP 8"
STUMP 15"
CONC.
FTN
E OUTLET
SLAB
SLAB GAR
SINK
UBX
WALL/FENCE
POLE
PUMP
HB
Property Line

VCRETE
CURB
WALL
DIMENSION POINT
CONC.
WALL/FENCE
CONCRETE
WALL/FENCE
N 55°30'00" E 30.30'
29.0'
30.6'
7.2'
Property Line







LEGEND

———	PROPERTY LINE
CB	CATCH BASIN
CC C&G	CONCRETE CURB & GUTTER
COL	COLUMN
CONC.	CONCRETE
EM	ELECTRIC METER
EOUTLET	ELECTRIC OUTLET
FF	FINISH FLOOR
FH	FIRE HYDRANT
FL	FLOWLINE
FTN	FOUNTAIN
GM	GAS METER
GP	GATE POST
HB	HOSE BIB
ICBX	IRRIGATION BOX
INV.	INVERT
JP	JOINT UTILITY POLE
LG	LIP OF GUTTER
MB	MAILBOX
SSCO	SANITARY SEWER CLEANOUT
SSMH	SANITARY SEWER MANHOLE
TC	TOP OF CURB
THRESH	THRESHOLD
TW	TOP OF WALL
UBX	UTILITY BOX
WM	WATER METER
WV	WATER VALVE
●12" TREE	TREE W/ SIZE
-X-X-	FENCE
-G-	GAS LINE
-OH-	OVERHEAD LINE
-SS-	SANITARY SEWER LINE
-W-	WATER LINE

UTILITY NOTE:

THE UTILITIES EXISTING ON THE SURFACE AND SHOWN ON THIS DRAWING HAVE BEEN LOCATED BY FIELD SURVEY. ALL UNDERGROUND UTILITIES SHOWN ON THIS DRAWING ARE FROM RECORDS OF THE VARIOUS UTILITY COMPANIES AND THE SURVEYOR/ENGINEER DOES NOT ASSUME RESPONSIBILITY FOR THEIR COMPLETENESS, INDICATED LOCATION, OR SIZE. RECORD UTILITY LOCATION SHOULD BE CONFIRMED BY EXPOSING THE UTILITY.

EASEMENT NOTE:

EASEMENTS, IF ANY, ARE NOT INDICATED HEREON.

FLOOD ZONE NOTE:

THE SUBJECT PROPERTY LIES ENTIRELY WITHIN FLOOD ZONE "X", BASED ON FLOOD INSURANCE RATE MAP 06081C0134F, DATED APRIL 05, 2019.

LOT AREA:

= 12,447 SQ. FT. ±
= 0.286 ACRES ±

GRAPHIC SCALE

(IN FEET)
1 inch = 10 ft.

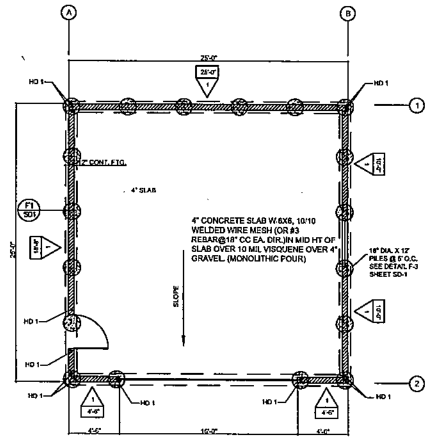
NO.	DESCRIPTION	BY	DATE

DAINS LAND SURVEYING
robin@dainslandsurveying.net
(650) 743-0831

PREPARED FOR:
ANGELO COSENTINO

TOPOGRAPHIC SURVEY PLAN
2896 HILLSIDE DRIVE
A.P.N. 027-112-280
DOCUMENT NO. 2022-017399
BURLINGAME SAN MATEO COUNTY CALIFORNIA

DRAWN BY:	RJD
DESIGNED BY:	---
CHECKED BY:	RJD
SCALE:	1"=10'
DATE:	12-06-23
PROJECT NO.	23-1366
SHEET	1 OF 1



FOUNDATION PLAN

BRACED WALLS SCHEDULE						
[illegible]	SHEATHING	EDGE NAILING	ANCHOR BOLTS	FIELD NAILING	ROOF SHEAR TRANSFER	
1	[illegible]	[illegible] O.C.	[illegible] O.C.	[illegible] @ 12" O.C.	[illegible]	
HD1	[illegible] (SEE DETAIL F-3 SHEET SD-1)					

NOTES:

SOLID SHEATHING ON ALL WALLS

PROVIDE [illegible] PLATE WASHERS FOR ALL A.B.s

FIELD NAILING [illegible] @ 12" O.C.

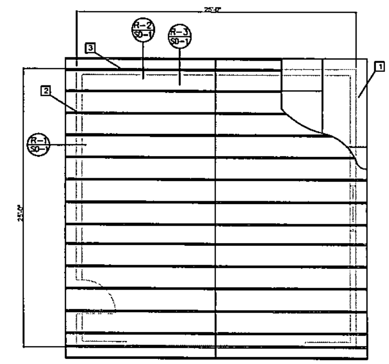
INDICATES BRACED WALL ABOVE

SEE BRACED WALL SCHEDULE

LENGTH OF WALL (FT-IN)

	NAILING SCHEDULE	
	CONNECTION	NAILING[f]
1	JOIST TO SILL OR GIRDER, TOENAIL	3-8d COMMON
2	BRIDGING TO JOIST, TOENAIL EACH END	2-8d COMMON
3	1"X6" SUB FLOOR OR LESS TO EACH JOIST, FACE NAIL	2-8d COMMON
4	WIDER THAN 1"X8" SUB FLOOR TO EACH JOIST, FACE NAIL	3-8d COMMON
5	2" SUB FLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL	2-16d COMMON
6	SOLE PLATE TO JOIST OR BLOCKING TYPICAL FACE NAIL SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL PANELS	16d @ 16" O.C 3-16d @ 16" O.C
7	TOP PLATE TO STUD, END NAIL	16d @ 24" O.C
8	STUD TO SOLE PLATE, TOE NAIL STUD TO SOLE PLATE, END NAIL	4-8d COMMON 2-16d COMMON
9	DOUBLE STUDS, FACE NAIL	16D @ 24" O.C
10	DOUBLE TOP PLATES, TYPICAL FACE NAIL DOUBLE TOP PLATES, LAP SPLICE	16d @ 16" O.C 8-16d COMMON
11	BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOENAIL	3-8d COMMON
12	RIM JOIST TO TOP PLATE, TOENAIL	8d @ 6" O.C
13	TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS, FACE NAIL	2-16d COMMON
14	CONTINUOUS HEADER, TWO PIECES, 16" O.C. ALONG EACH EDGE	16d COMMON
15	CEILING JOISTS TO PLATE, TOENAIL	3-8d COMMON
16	CONTINUOUS HEADER TO STUD, TOENAIL	4-8d COMMON
17	CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL	3-16d COMMON
18	CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL	3-16d COMMON
19	RAFTER TO PLATE, TOENAIL	2-16d COMMON
20	1" DIAGONAL BRACE TO EA. STUD AND PLATE, FACE NAIL	2-8d COMMON
21	1"X8" SHEATHING OR LESS TO EACH BEARING, FACE NAIL	3-8d COMMON
22	WIDER THAN 1"X8" SHEATHING TO EACH BEARING, FACE NAIL	3-8d COMMON
23	BUILT-UP CORNER STUDS, 24" O.C.	16d COMMON
24	BUILT-UP GIRDER AND BEAMS, FACE NAIL @ 32" O.C. TOP 4 BOTTOM AND STAGGERED ON OPPOSITE SIDES FACE NAIL AT ENDS AND AT EACH SPLICE	 20d COMMON 2-20d COMMON
25	2 PLANKS @ EACH BEARING	2-16d COMMON
26	COLLAR TIE TO RAFTER, FACE NAIL	3-10d COMMON
27	JACK RAFTER TO HIP, TOENAIL FACE NAIL	3-10d COMMON 2-16d COMMON
28	ROOF RAFTER TO 2 X RIDGE, TOENAIL FACE NAIL	4-16d COMMON 3-16d COMMON
29	JOIST TO BAND JOIST, FACE NAIL	3-16d COMMON
30	LEDGER STRIP, FACE NAIL AT EACH JOIST OR RAFTER	3-16d COMMON
31	WOOD STRUCTURAL PANELS AND PARTICALBOARD[b] SUB FLOOR AND WALL SHEATHING (TO FRAMING) 1/2" AND LESS 19/32" - 3/4" 7/8" - 1" SINGLE FLOOR (COMBINATION SUBFLOOR-UNDERLAYMENT TO FRAMING): 3/4" AND LESS 7/8" - 1" 1-1/8" - 1-1/4"	 6d[f] 8d[d] OR 6d[f] 8d[f] 10d[f] OR 8d[f] 6d[f] 8d[f] 10d[d] OR 8d[f]
32	PANEL SIDING (TO FRAMING): 1/2" OR LESS 5/8"	 6d 8d
33	FIBERBOARD SHEATHING[g] 1/2" 25/32"	NO. 11 GA. ROOFING NAIL 6d COMMON NO. 16 GA. STAPLE 8D COMMON NO. 16 GA. STAPLE
34	INTERIOR PANELING 1/4" 3/8"	 4d 6d

a. Common or box nails are permitted to be used except where otherwise stated.
b. Nails spaced at 6 inches on center at edges, 12 inches at intermediate supports except 6 inches at all supports where spans are 48 inches or more. For nailing of wood structural panel and particleboard diaphragms and shear walls, refer to Sections 2305. Nails for wall sheathing may be common, box or casing.
c. Common or deformed shank
d. Common
e. Deformed shank
f. Corrosion-resistant siding or casing nails.
g. Fasteners spaced at 3 inches on center at exterior edges and 6 inches on center at intermediate supports, when used as structural sheathing. Spacing shall be 6 inches on center on the edges and 12" on center at intermediate supports for non-structural applications
h. Corrosion-resistant roofing nails with 1/16 inch-diameter head and 1 1/2 inch length for 1/2 inch sheathing and 1 3/4 inch length for 25/32 inch sheathing conforming to the requirements of Section 2304.3
i. Corrosion-resistant staples with nominal 1/16 inch crown and 1 1/8 inch length for 1/2 inch sheathing and 1 1/2 inch length for 25/32 inch sheathing. Panel supports at 16 inches (20 inches if strength axis in the long direction of the panel, otherwise marked.)
j. Casing or finish nails spaced 6 inches on panel edges, 12 inches at intermediate supports
k. Panel supports at 24 inches. Casing or finish nails spaced at 6 inches on panel edges, 12 inches at intermediate supports.
l. For roof sheathing applications, 8d nails are the minimum required for wood structural panels
m. Staples shall have a minimum crown width of 1/16 inch.
n. For roof sheathing applications, fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports.
o. Fasteners spaced 4 inches on center at edges, 6 inches at intermediate supports for subfloor and wall sheathing and 3 inches on center at edges, 6 inches at intermediate supports for roof sheathing.
p. Fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports.



ROOF FRAMING PLAN

ROOF FRAMING KEYNOTES: #	
1	ROOF SHEATHING: "⅜" MIN APA RATED 24 / 16 MINIMUM PANEL SPAN SHEATHING NAILED WITH 8d @ 6" C.C. EDGE AND 12" C.C. FIELD. – STAGGER JOINTS THAT OCCUR OVER FRAMING MIN OF 24" – SHEATHING TO BE CONTINUOUS BELOW FRAME OVER AREAS. – MAY HAVE 22" X 30" OPENING FOR VENTILATION AND ACCESS. BLOCK AND EDGE NAIL AROUND PERIMETER OF OPENING
2	PLACE SIMPSON H1 AT ALL TRUSSES TO BEARING WALLS OR BEAMS. IF 2-PLY TRUSS USE SIMPSON H10-2 OR H11-Z. IF 3-PLY OR MORE PLY TRUSS, USE PAIR OF SIMPSON TSP (ONE EACH SIDE OF TRUSS). TYPICAL UNO ON PLAN.
3	SIMPSON HTS30 TOP PLATE TO PARALLEL TRUSS BOTTOM CHORD.
GEN	TRUSS TO TRUSS HANGERS PER TRUSS MANUFACTURES SPECIFICATIONS.

DESIGN/BUILDER:

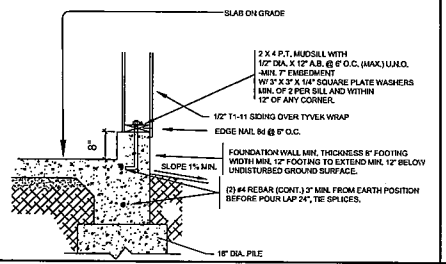
PROJECT:

COSENTINO RESIDENCE
2896 HILLSIDE DRIVE
BURLINGAME, CA 94010

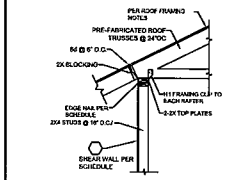
APN: 027-112-280

No.	Revision/Issue	Date

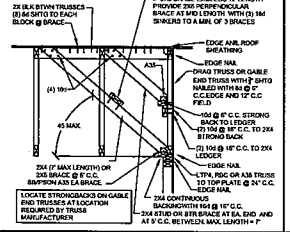
Project:
Sheet: A2
Date: 8/14/2024
Scale: VARIES



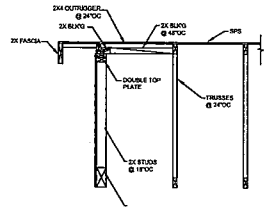
F-1 FOUNDATION (PIER) DETAIL NTS



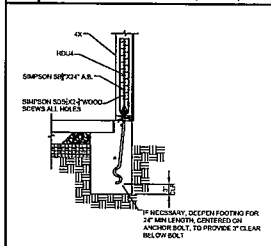
R-1 ROOF TO WALL DETAIL



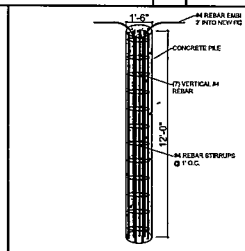
R-2 SHEAR TRANSFER DETAIL



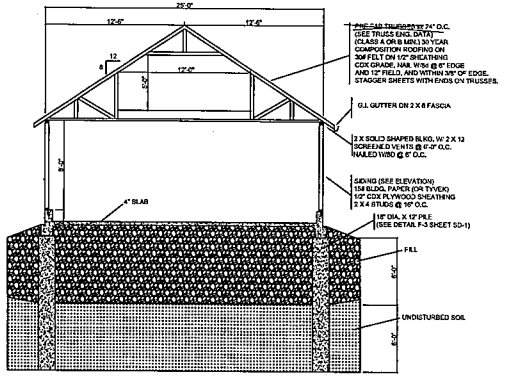
R-3 RAKE EAVE SUPPORT DETAIL



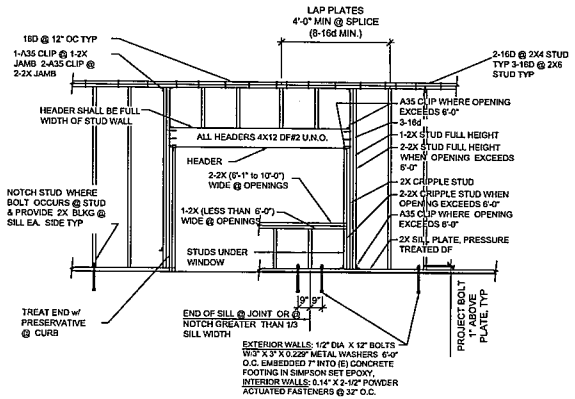
F-2 HOLD DOWN DETAIL



F-3 PILE DETAIL



SECTION A



TYPICAL STUD WALL FRAMING NTS

DESIGN/BUILDER:

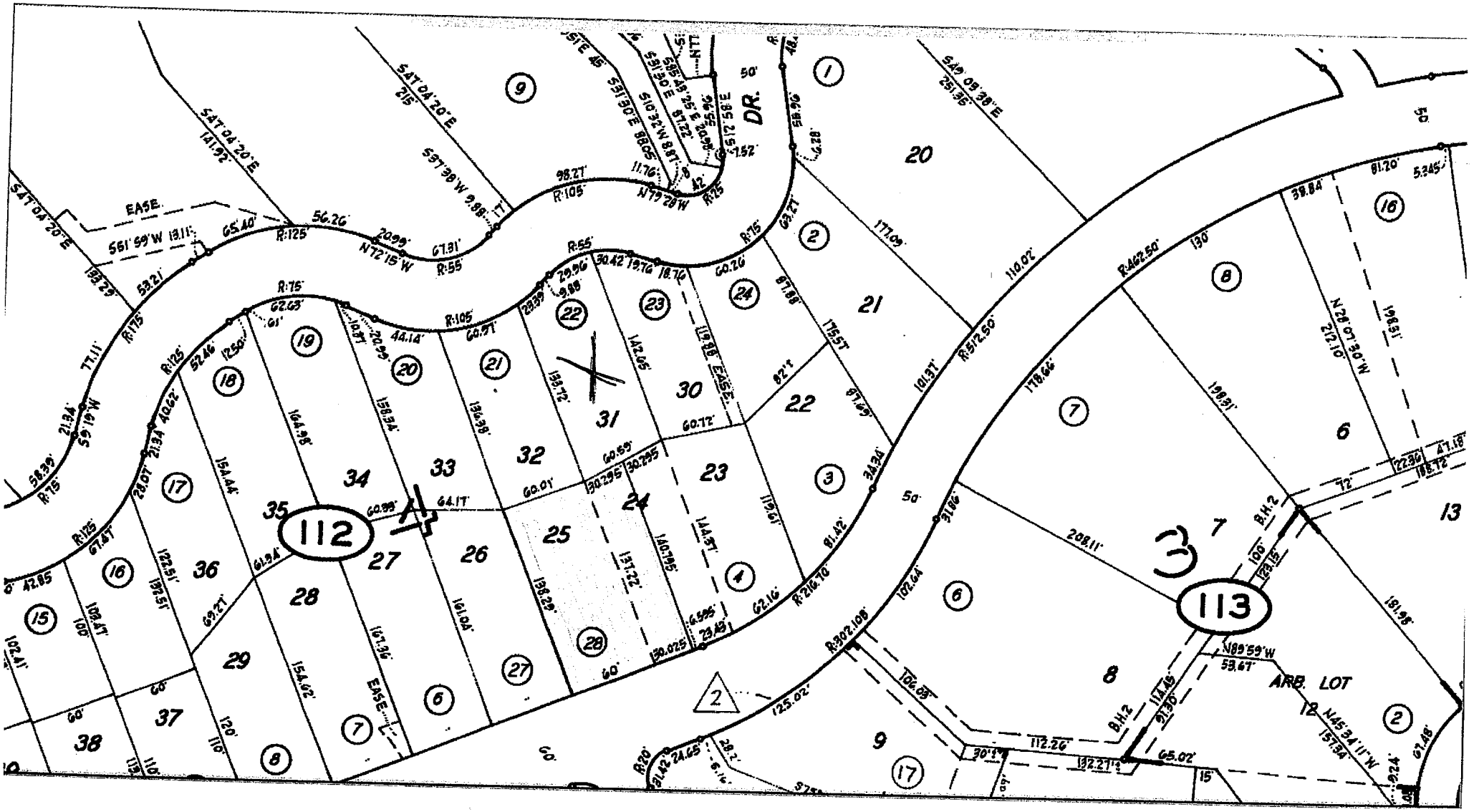
PROJECT:

COSENTINO RESIDENCE
2896 HILLSIDE DRIVE
BURLINGAME, CA 94010

APN: 027-112-280

No.	Revision/Issue	Date

Project
Date 8/14/2024
Scale 1/4" = 1'
Sheet SD1


DR.
112
113
ARB. LOT
EASE
EASE
EASE
S61°59'W 13.11'
S61°30'E 88.06'
S10°32'W 8.87'
S85°49'25"E 87.22'
S61°30'E
S.A.T. 04°20'E
S.A.T. 04°20'E 141.92'
S.A.T. 04°20'E 215'
S.A.T. 04°20'E
S.A.T. 05°38'E 251.35'
S97°38'W 9.98'
N79°28'W
N72°15'W
N.45°34'11"W 157.39'
N89°59'W 53.67'
N.28°07'30"W 212.10'
R:105'
R:125'
R:75'
R:55'
R:175'
R:462.50'
R:512.50'
R:216.70'
R:302.108'
R:20'
98.27'
65.40'
56.26'
67.31'
77.11'
53.21'
59.19'W
58.99'
21.34'
40.62'
67.47'
42.85'
102.41'
100.47'
122.51'
69.27'
154.42'
161.26'
161.04'
138.29'
131.22'
140.795'
144.37'
119.61'
62.16'
81.42'
34.34'
101.37'
110.02'
171.09'
175.57'
97.88'
82.11'
60.72'
60.65'
60.01'
64.17'
60.99'
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60.27'
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120'
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208.11'
102.64'
91.86'
112.26'
132.27'
65.02'
106.85'
114.45'
21.36'
125.02'
198.91'
178.66'
181.98'
67.48'
81.20'
5.245'
38.84'
47.18'
22.36'
165.72'
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Home CONTRACTORS STATE LICENSE BOARD

Contractor's License Detail for License # 1046198

DISCLAIMER: A license status check provides information taken from the CSLB license database. Before relying on this information, you should be aware of the following limitations.

- CSLB complaint disclosure is restricted by law (B&P 7124.6) If this entity is subject to public complaint disclosure click on link that will appear below for more information. Click here for a definition of disclosable actions.
- Only construction related civil judgments reported to CSLB are disclosed (B&P 7071.17).
- Arbitrations are not listed unless the contractor fails to comply with the terms.
- Due to workload, there may be relevant information that has not yet been entered into the board's license database.

Data current as of 3/11/2025 9:18:16 PM

Business Information

A COSENTINO BUILDING AND RENOVATION INC
P O BOX 1845
BURLINGAME, CA 94011
Business Phone Number:(650) 218-1362

Entity Corporation
Issue Date 10/26/2018
Expire Date **10/31/2022**

License Status

This license is expired and not able to contract at this time.

Additional Status

- Before the license can be renewed active or reactivated, the corporation's status at the Secretary of State's Office must be returned to active.
- The license will need a contractors bond to renew active or reactivate.
- The license will need to meet the workers compensation requirements to renew active or reactivate.

Classifications

B - GENERAL BUILDING

Drainage Report

for the

Cosentino Residence at

2896 Hillside Drive

Burlingame (SMCO), CA 94010

Prepared By:

Travis R. Lutz, PE, QSD/QSP

December 10, 2024

Introduction

This Drainage Report has been prepared in accordance with the County of San Mateo *Guidelines for Drainage Review*. The report consists of hydrological analysis for the proposed development to ensure the storm drain system has been adequately designed for the design storm event.

The project consists of construction of new garage, and associated flatwork which creates or replaces an area approximately 2,900 square foot. Because of this, the Rational Formula was used to determine the peak flow rate using an Intensity provided by NOAA data. Due to the sites size the time of concentration is well under 10 minutes, so the 10 minute minimum time of concentration was used per County standards.

Existing Area and Site Drainage Conditions

Current site runoff drains via overland flow in the northeasterly and southeasterly direction. The existing site has a composite run-off coefficient of 0.59. The sites existing impervious area is 6,018 square feet. The total lot run-off rate for the 10-year storm event was found to be 0.386 cfs.

Proposed Site Drainage Condition

Proposed runoff will continue to drain towards the historic drainage patterns and the sites natural low points, via overland flow in the northeasterly and southeasterly direction. The proposed site has a composite run-off coefficient of 0.56. The sites proposed impervious area is 5,491 square feet. The total lot run-off rate for the 10-year storm event was found to be 0.37 cfs. A trench dissipater has been designed to retain the created/replaced impervious areas based on County standards.

Storm Water Management Plan

The proposed project design utilizes several design elements in an effort to provide Better Management Practices and control and manage storm water. The design elements include;

- Direct runoff from walkways and/or patios onto vegetated areas.
- Construct driveway with pervious surfaces.

Summary

The hydrological calculations in this report show that runoff has been returned to pre-construction conditions. Upstream and downstream drainage patterns have been restored to pre-construction conditions per State law and County criteria.

NOAA Atlas 14, Volume 6, Version 2
Location name: Burlingame, California, USA*
Latitude: 37.5788°, Longitude: -122.388°
Elevation: 377 ft**
* source: ESRI Maps
** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour)[1]

Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	1.81 (1.58-2.09)	2.21 (1.93-2.54)	2.74 (2.39-3.18)	3.20 (2.76-3.74)	3.86 (3.19-4.72)	4.39 (3.53-5.52)	4.97 (3.86-6.43)	5.58 (4.20-7.50)	6.48 (4.62-9.18)	7.24 (4.94-10.7)
10-min	1.30 (1.14-1.50)	1.58 (1.38-1.82)	1.96 (1.71-2.27)	2.29 (1.98-2.69)	2.77 (2.29-3.38)	3.15 (2.53-3.95)	3.56 (2.77-4.61)	4.00 (3.01-5.38)	4.64 (3.31-6.58)	5.18 (3.54-7.66)
15-min	1.05 (0.920-1.21)	1.27 (1.12-1.47)	1.58 (1.38-1.84)	1.85 (1.60-2.16)	2.23 (1.84-2.73)	2.54 (2.04-3.19)	2.87 (2.24-3.72)	3.23 (2.43-4.34)	3.74 (2.67-5.30)	4.18 (2.86-6.18)
30-min	0.726 (0.636-0.836)	0.882 (0.772-1.02)	1.10 (0.956-1.27)	1.28 (1.11-1.50)	1.55 (1.28-1.89)	1.76 (1.41-2.21)	1.99 (1.55-2.58)	2.24 (1.68-3.00)	2.60 (1.85-3.68)	2.90 (1.98-4.28)
60-min	0.514 (0.451-0.592)	0.625 (0.547-0.721)	0.777 (0.677-0.900)	0.907 (0.783-1.06)	1.09 (0.903-1.34)	1.24 (1.00-1.56)	1.41 (1.10-1.82)	1.58 (1.19-2.13)	1.84 (1.31-2.60)	2.05 (1.40-3.03)
2-hr	0.377 (0.330-0.434)	0.455 (0.398-0.526)	0.562 (0.490-0.652)	0.653 (0.563-0.765)	0.782 (0.646-0.956)	0.885 (0.711-1.11)	0.994 (0.774-1.29)	1.11 (0.836-1.49)	1.28 (0.913-1.81)	1.42 (0.970-2.10)
3-hr	0.314 (0.275-0.362)	0.380 (0.333-0.439)	0.469 (0.409-0.544)	0.545 (0.470-0.638)	0.651 (0.538-0.796)	0.736 (0.592-0.925)	0.826 (0.643-1.07)	0.922 (0.693-1.24)	1.06 (0.755-1.50)	1.17 (0.800-1.73)
6-hr	0.222 (0.195-0.256)	0.271 (0.237-0.313)	0.337 (0.294-0.390)	0.392 (0.338-0.459)	0.469 (0.388-0.574)	0.531 (0.427-0.667)	0.595 (0.464-0.772)	0.664 (0.499-0.892)	0.761 (0.543-1.08)	0.840 (0.574-1.24)
12-hr	0.144 (0.126-0.166)	0.180 (0.157-0.208)	0.228 (0.199-0.264)	0.268 (0.231-0.314)	0.324 (0.268-0.396)	0.368 (0.296-0.463)	0.414 (0.323-0.537)	0.463 (0.348-0.623)	0.532 (0.380-0.754)	0.588 (0.401-0.869)
24-hr	0.093 (0.085-0.104)	0.119 (0.108-0.133)	0.153 (0.139-0.172)	0.181 (0.163-0.205)	0.221 (0.192-0.259)	0.252 (0.214-0.302)	0.284 (0.236-0.349)	0.318 (0.257-0.403)	0.366 (0.283-0.483)	0.404 (0.301-0.553)
2-day	0.060 (0.054-0.067)	0.076 (0.069-0.085)	0.096 (0.088-0.109)	0.114 (0.102-0.129)	0.138 (0.120-0.162)	0.157 (0.134-0.189)	0.177 (0.147-0.218)	0.198 (0.160-0.251)	0.228 (0.176-0.301)	0.251 (0.187-0.344)
3-day	0.046 (0.042-0.052)	0.058 (0.053-0.065)	0.074 (0.067-0.083)	0.087 (0.078-0.099)	0.105 (0.091-0.124)	0.120 (0.102-0.144)	0.135 (0.112-0.165)	0.150 (0.121-0.190)	0.172 (0.133-0.227)	0.189 (0.141-0.259)
4-day	0.039 (0.035-0.043)	0.049 (0.044-0.054)	0.061 (0.056-0.069)	0.072 (0.065-0.082)	0.087 (0.075-0.102)	0.099 (0.084-0.118)	0.111 (0.092-0.136)	0.123 (0.099-0.156)	0.141 (0.109-0.186)	0.155 (0.115-0.212)
7-day	0.027 (0.025-0.031)	0.034 (0.031-0.039)	0.043 (0.039-0.049)	0.051 (0.046-0.058)	0.061 (0.053-0.071)	0.069 (0.058-0.083)	0.077 (0.064-0.095)	0.086 (0.069-0.108)	0.097 (0.075-0.129)	0.107 (0.080-0.146)
10-day	0.022 (0.020-0.024)	0.027 (0.025-0.030)	0.034 (0.031-0.039)	0.040 (0.036-0.045)	0.048 (0.042-0.056)	0.054 (0.046-0.065)	0.060 (0.050-0.074)	0.067 (0.054-0.085)	0.076 (0.058-0.100)	0.083 (0.062-0.113)
20-day	0.014 (0.013-0.016)	0.018 (0.016-0.020)	0.022 (0.020-0.025)	0.026 (0.023-0.030)	0.031 (0.027-0.036)	0.035 (0.029-0.042)	0.038 (0.032-0.047)	0.042 (0.034-0.053)	0.047 (0.036-0.062)	0.051 (0.038-0.070)
30-day	0.011 (0.010-0.013)	0.014 (0.013-0.016)	0.018 (0.016-0.020)	0.021 (0.019-0.024)	0.025 (0.022-0.029)	0.028 (0.023-0.033)	0.030 (0.025-0.037)	0.033 (0.027-0.042)	0.037 (0.028-0.049)	0.040 (0.029-0.054)
45-day	0.009 (0.008-0.010)	0.012 (0.011-0.013)	0.015 (0.013-0.017)	0.017 (0.015-0.020)	0.020 (0.017-0.024)	0.022 (0.019-0.027)	0.024 (0.020-0.030)	0.026 (0.021-0.033)	0.029 (0.022-0.038)	0.031 (0.023-0.042)
60-day	0.008 (0.008-0.009)	0.011 (0.010-0.012)	0.013 (0.012-0.015)	0.015 (0.014-0.017)	0.018 (0.015-0.021)	0.020 (0.017-0.023)	0.021 (0.017-0.026)	0.023 (0.018-0.029)	0.025 (0.019-0.033)	0.026 (0.019-0.036)

[1] Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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PF graphical

Hydrological Calculations

Existing Conditions

Design Storm: 10 yr

Time of Concentration:

T_c = 10.00 min

Intensity:

Intensity = 2.29 in/hr (from NOAA)

Area:

Impervious Area = 6018 ft^2 C=0.9

Pervious Area = 6429 ft^2 C=0.3

Total Area = 0.29 acres

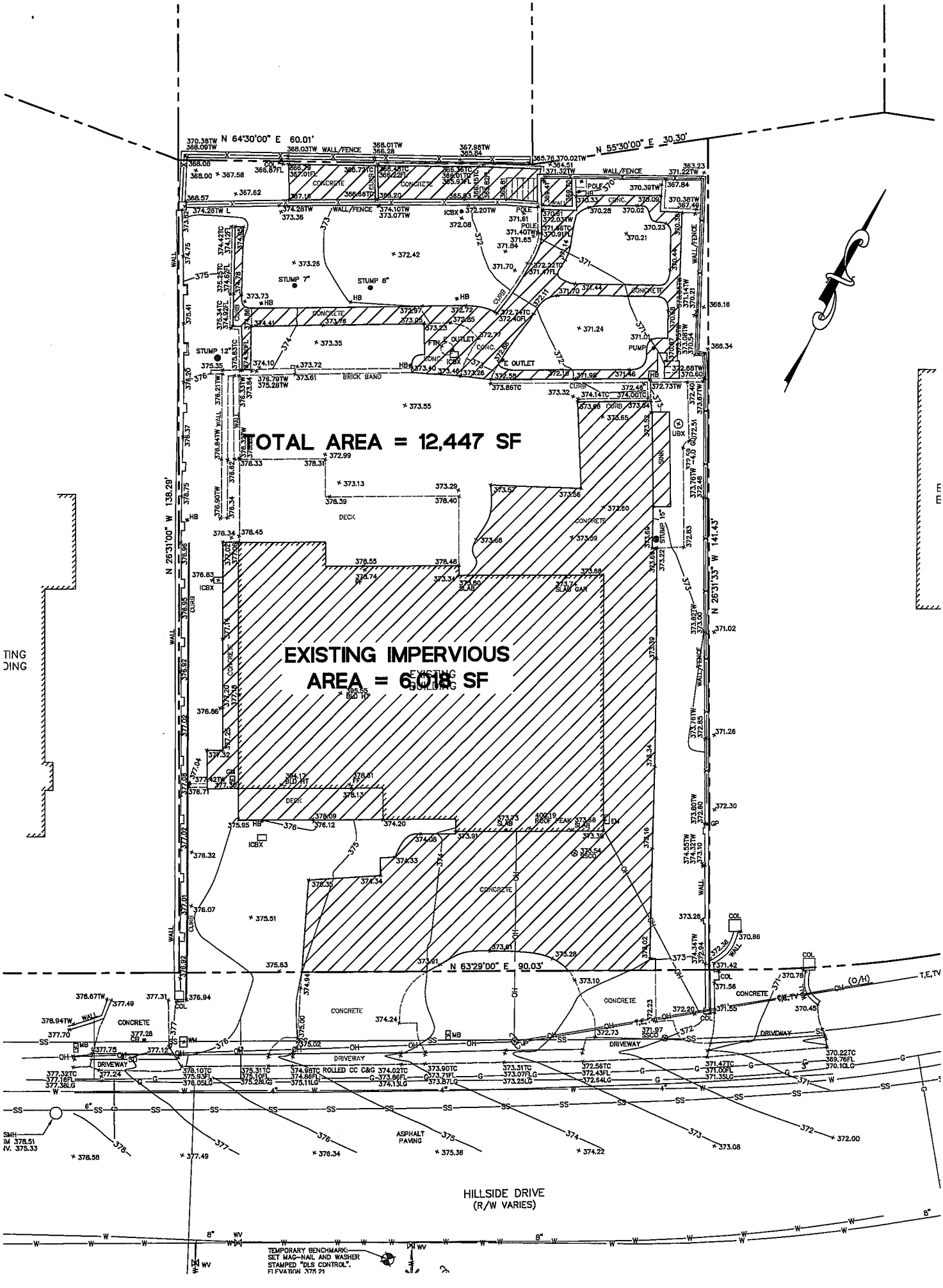
Runoff Coefficient:

C_W= (6018/12447) x (0.9) + (6429/12447) x (0.3)

Weighted C = 0.59 %

Discharge: *Q=CIA*

Q= 0.386 cfs


N 64°30'00" E 60.01'
N 55°30'00" E 30.30'
N 26°31'00" W 138.29'
N 26°31'35" W 141.43'
N 63°29'00" E 90.03'
TOTAL AREA = 12,447 SF
EXISTING IMPERVIOUS
AREA = 6,018 SF
EXISTING BUILDING
DECK
CONCRETE
DRIVEWAY
ASPHALT PAVING
HILLSIDE DRIVE
(R/W VARIES)
TEMPORARY BENCHMARK:-
SET MAG-NAIL AND WASHER
STAMPED "DLS CONTROL".
ELEVATION 375.21

Hydrological Calculations

Proposed Conditions

Design Storm: 10 yr

Time of Concentration:

T_c = 10.00 min

Intensity:

Intensity = 2.29 in/hr (from NOAA)

Area:

Impervious Area = 5491 ft^2 C=0.9

Pervious Area = 6956 ft^2 C=0.3

Total Area = 0.29 acres

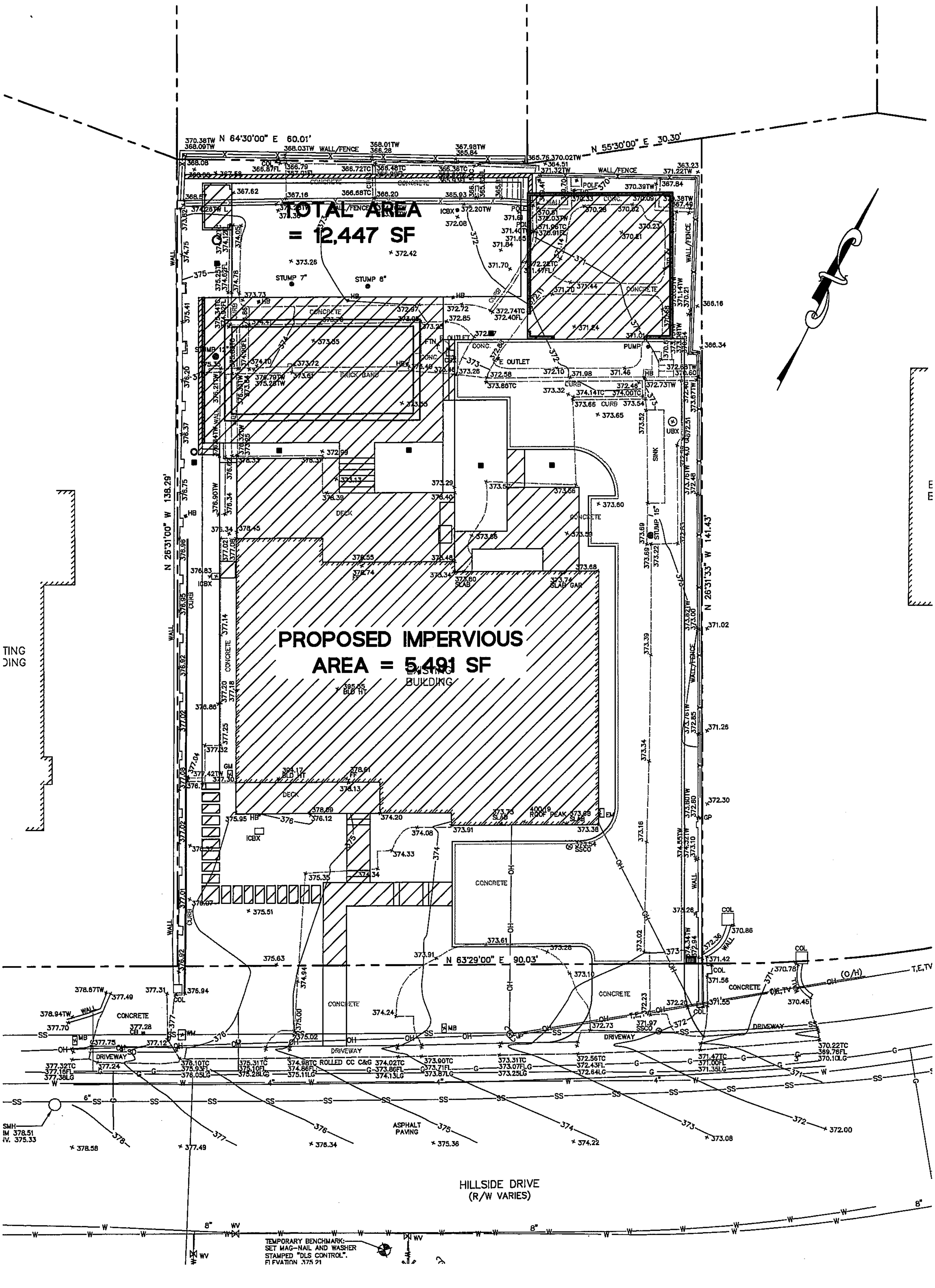
Runoff Coefficient:

C_W= (5491/12447) x (0.9) + (6956/12447) x (0.3)

Weighted C = 0.56 %

Discharge: *Q=CIA*

Q= 0.370 cfs


TOTAL AREA
= 12,447 SF
PROPOSED IMPERVIOUS
AREA = 5,491 SF
EXISTING
BUILDING
N 64°30'00" E 60.01'
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N 26°31'00" W 138.29'
N 26°31'33" W 141.43'
N 63°29'00" E 90.03'
HILLSIDE DRIVE
(R/W VARIES)
ASPHALT
PAVING
DRIVEWAY
CONCRETE
DECK
WALL/FENCE
TEMPORARY BENCHMARK:
SET MAG-NAIL AND WASHER
STAMPED "DLS CONTROL".
ELEVATION 375.21

Summary

Existing Discharge:

Q= 0.386 cfs

Proposed Discharge:

Q= 0.370 cfs

Detention Volume Required:

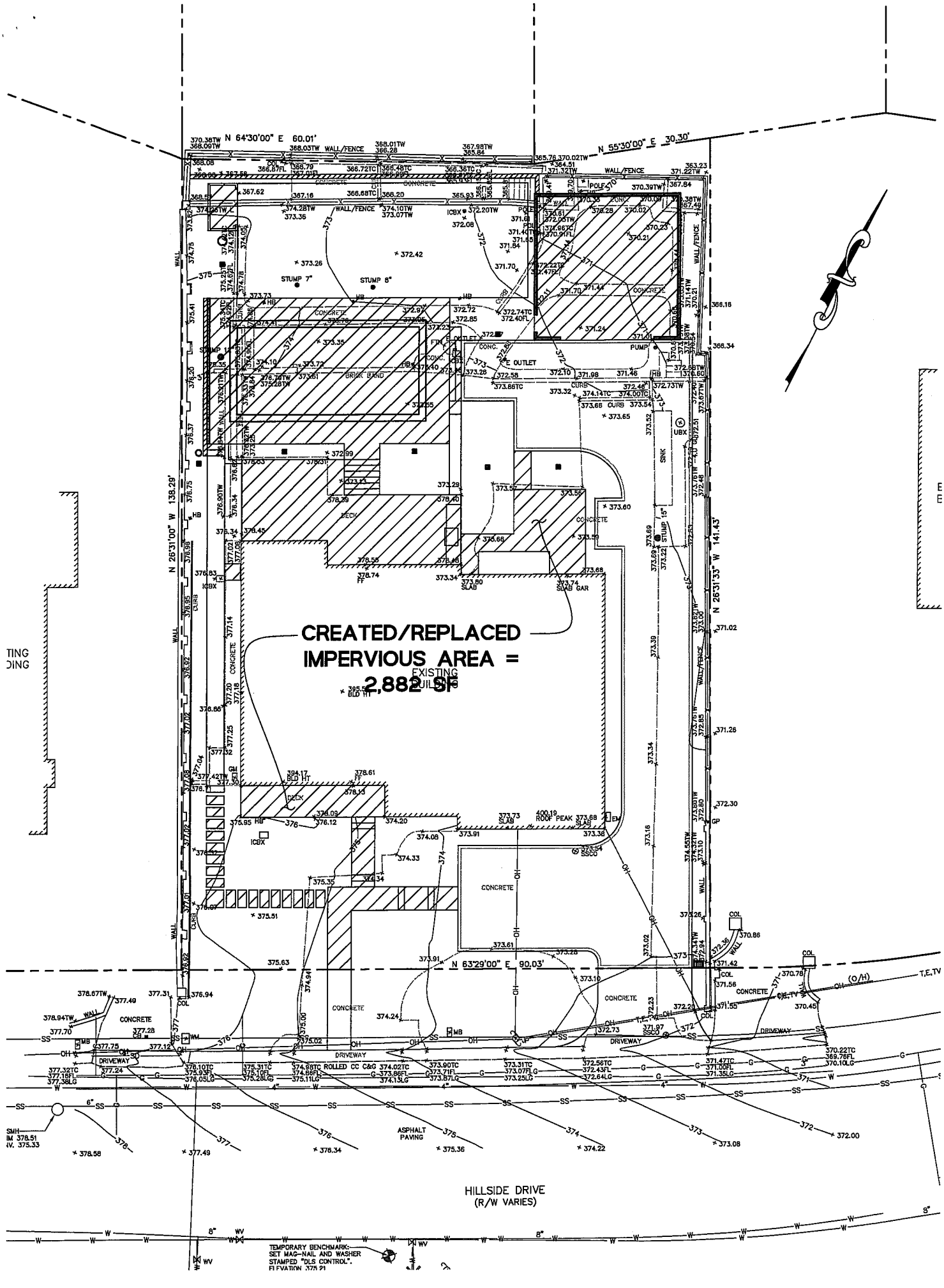
$Q_{delta} = A_{impervious} \times 0.6 \times i_{1\text{-hour}}$ 0.036 cfs

Detain difference in Q for a 60 minute duration

Q_{delta}*60 minutes*1.5 safety factor
194.43 cf

Detention Volume Proposed:

Pipe I.D. =	36	Pipe Lengt	15.00	105.98	cf
Pipe O.D. =	42				
Drain Rock Void Area	35%	of Drain Rock Volume			
Trench Volume =				445.50	cf
Pipe Volume O.D. =				144.24	cf
Drain Rock Volume =				105.44	cf
Total Detention Volume Provided =				211.41	cf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N 26°31'00" W 138.29'
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N 63°29'00" E 90.03'
CREATED/REPLACED
IMPERVIOUS AREA =
2,882 SF
EXISTING
BUILDING
HILLSIDE DRIVE
(R/W VARIES)
TEMPORARY BENCHMARK:
SET MAG-NAIL AND WASHER
STAMPED "DLS CONTROL".
ELEVATION 375.91
DECK
CONCRETE
DRIVEWAY
ASPHALT
PAVING
SINK
PUMP
SLAB
ROOF PEAK
STUMP 7"
STUMP 8"
STUMP 15"
SMH
IM 378.51
W. 375.33